#14 Barron Trail 37 Bridge 12417

State of Wisconsin	
Department of Natural Resources	
dnr.wi.gov	

Motorized Recreation Grant Application

For: (choose all that apply)

Form 8700-159 (R 02/2024) Page 1 of 5

Due Date: April 15

ATV/UTV Trail Aid Snowmobile Trail Aid

Notice: Completion of this form is required under Wisconsin Statutes 23.09(26) and 23.33. Failure to complete this form will result in denial of financial assistance. Personally identifiable information found on this form is not intended to be used for any other purpose. The Department of Natural Resources (DNR) may provide this information to requesters as required by Wisconsin's Public Records law {ss. 19.31 – 19.39, Wis. Stats.}.

Instructions: Applications may con	nbine mo	re than	one sou	rce of fui	nds. They	may	D	NR Use	e Only	
be submitted for consideration of tra Stewardship funding. Submit one co necessary attachments. Send appli	aditional A opy of all cations to	ATV, U forms a your <u>C</u>	TV, Snow and attack Communit	vmobile a hments. ty Service	and Motor See Page es Specia	ized 2 for <u>llist</u> .	Category			Number
Section 1: Applicant Information	n									
Applicant / Organization Name				C	heck Red	cipient: In	dividual other than	n author	rized inc	dividual to act
Jeff Wolfe/Barron County				0	n behalf c	of the app	llicant. 🗌 Se	lect if th	ne same	e as applicant.
Individual Authorized to Act on Beh	alf of App	licant p	er Resol	ution C	Check Re	cipient N	lame (Name to A	opear o	n Chec	k)
Jeff Wolfe				J	odi Busc	h				
Title				Т	ītle					
Deputy Sheriff				F	Finance I	Director				
Address				A	Address					
1420 Highway 25 N.				3	335 E. M	onroe A	venue			
City		State	ZIP Code	e C	City				State	ZIP Code
Barron		WI	548	12 E	Barron				WI	54812
Telephone Number		Email /	Address							
(715) 637-6710		jeff.w	olfe@co	barron.	.wi.us					
Section 2: Project Information I	Required	for all	Project	S						
Project Title						Current	t Funded Miles	New N	liles (if	applicable)
Trail 37 Bridge Rehab						337.60				
County	Township	Rang	ge OE	Section	1/4 1/4	1/4	GPS Coordinate Lat. 45.3400	s: 013		
Barron	33 N	J 12	2	17	SE	SE	Long91.873	327		

Project Description Summary

In March of 2024, Barron County had this bridge inspected by CORRE Inc. The inspection stated that the support pilings had failed and needed to be replaced. The inspector also recommended replacing the bridge decking and railings as they were nearing the end of life. Barron County proposes replacing the support pilings, decking and railings. The current and future load ratings are being calculated by CORRE Inc. and the inspection report will be provided when received.

X I certify that all maintenance land use agreements are on file.

Estimated Cost						
Maintenance	Acquisition	Insurance	Development	Bridge Rehab.	Trail Rehab.	Total Estimated Cost
				\$122,600.00		\$122,600.00
		Lea	ave Blank – DNR	Use Only		
Applicant Certi	fication					
Printed Name of	Authorized Official		Of	ficial's Title		
Jeff Wolfe			De	eputy Sheriff	2 ²	
As the applicant	's authorized official	I certify that, to the	e best of my know	ledge, the informatio	on in this application	n is true and correct.

Signature of Adherized Official

Date Prepared

Motorized Recreation Grant Application

Jeff Wolfe/Barron County

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Appendix A – Required for	Bridge F	Rehab	/Repla	ace, N	ew, or F	Reroute	e with New Br	idge
X Bridge Rehab/Replace	Nev	w Bridg	е	R	eroute w	ith new b	bridge	
County	Township	Range	Oe	Section	1/4 1/4	1/4	GPS Coordinate Lat. 45.3400	s: 014
Barron	33 N	12	ΘW	17	SE	SE	Long91.873	327
Water Body Name				Brid	ige Name	9		12417
Fourmile Creek					the instant		ver received deve	Incoment or rehabilitation funds
Funded Trail Name or Number (SN	ARS if app	olicable)		Has lin t	s this brid he past?		\sim No Ye	ear: 2015 \$ 6,816.00
37							$\frac{62.5'x12'}{x12'}$	· · · · · · · · · · · · · · · · · · ·
Bridge is located on: Private 	e property				Bridge/C		$\frac{62.5'x12}{2}$	5'
OPublic	property			Ne	w Bridge/	Sumber	Length of T	rail Use Agreement (5 year
Landowner Where Bridge is Locat	ed					4640	10	minimum)
Charles G. Carr/Brian R. Carr			Ago of	(/	Bride	4049 ne Materia	al 10	
Current maximum load 48,00	0	_lbs.	Age of	Bridge	Stop	.1		
Proposed maximum load 48,0	00	_lbs.	Unkno	own	Contact			Telephone Number
Sponsoring Club Name				Laff	Vorgin			(715) 837-1600
Barron Bears Snowmobile Clu	b	ovimur	n load?	What	is the ma	aximum lo	ad of the other b	ridges on the system if
Do you have your trail bridges pos		Yes	\bigcirc No	groor	ned with	this bridg	e?	
The state of the second s	drag/grad	ing equ	inment?	10,0	00 lbs-6	0,000 lbs	5	
What is the weight of your puller a	x ulag/glau	ing equ	ipment.					
24,251	are nlanne	d for thi	s bridae	2				
ATV/Snowmobiling			o bridge					
A I V/Showmouning	es planned	how m	uch of t	he brido	ie cost wi	ill be paid	for by non-snow	mobile or non-ATV users?
If there are other Recreational us	co plannou	,						
N/A						Createlist		n a permit?
Yes O No Have you cont	acted your	local <u>D</u>	<u>NR Wat</u>	er Mana	igement	<u>Specialist</u>		
● Yes ○ No Is a permit nee	eded? (Ple	ase pro	vide an	/ written	correspo		in determination	2
Yes No Have you cont	acted your	County	Zoning	Dept. re	egaraing			:
	(hydrologio	c and hy	draulic)	study b	e require	a?		

Bridge Project Detailed Description

In March of 2024, Barron County had this bridge inspected by CORRE Inc. The inspection stated that the support pilings had failed and needed to be replaced. The inspector also recommended replacing the bridge decking and railings as they were nearing the end of life. Barron County proposes remove the existing deck and railings, temporarily relocate the bridge structure, install new timber piles and timber piers, reinstall bridge structure, and then install a new panelized timber deck and railings.

State of Wisconsin Department of Natural Resources PO Box 7921, Madison WI 53707-7921 dnr.wi.gov

Recreation Grant Project Cost Estimate Worksheet

Page 1 of 2

For use with Recreation Grant Application Forms

Form 8700-014 (R 02/23)

	Prepared By:	Date
	Jeff Wolfe	04/09/2024
Project Applicant:	Landowner Name	O Public
Barron County	Charles & Brian Carr	 Private
	Project Applicant: Barron County	Prepared By: Jeff Wolfe Project Applicant: Barron County Charles & Brian Carr

	Indicate - (C) Contract , (F) Force Acct., (D) Donated				
\downarrow	DEVELOPMENT PROJECT ITEMS List by individual item or break down by Use Areas (See Item List On Back Of This Form)	Quantity	Unit of Measure	Component Costs	Estimated Total Item Cost
С	Crane Mobilization Fee	1	LS	\$22,500.00	22,500.00
С	Removal of existing bridge deck, railings, and moving bridge	1	LS	\$8,400.00	8,400.00
С	Removal of piers	4	Each	\$2,200.00	8,800.00
С	New open bent timber pier installation	2	Each	\$18,000.00	36,000.00
С	Installation of new panelized timber deck with railings	1	LS	\$145,000.00	145,000.00
С	Site grading	1	LS	\$9,500.00	9,500.00
C	Erosion control	1	LS	\$10,000.00	10,000.00
C	Engineering	1	LS	\$5,000.00	5,000.00
				TOTAL \$	\$245,200.00

NOTE:

- For development projects, contingency and indirect costs are <u>not</u> eligible expenses.
- For acquisition projects, complete the Acquisition Project Cost Estimate Section of this form.

CONSTRUCTION CO., INC.

715.723.9708

Larson Construction Co Inc

19681 - 55th Avenue Chippewa Falls WI 54729 PH 715.723.9708 FX 715.723.4515 Barron County Trail System Trail 136 Bridge Rehabilitation

		BASE BID				
Line No	Item No	Description	Quantity	Unit	Unit Price	Total
1		Mobilization	1.00	LS	22,500.00	\$ 22,500.00
2		Removal of Existing deck	1.00	LS	8,400.00	\$ 8,400.00
3		removal of piers	4.00	EA	2,200.00	\$ 8,800.00
4		New open bent timber pier installation	2.00	EA	18,000.00	\$ 36,000.00
5		Installation of new panelized timber deck with railing	1.00	LS	145,000.00	\$ 145,000.00
6		Site grading	1.00	LS	9,500.00	\$ 9,500.00
7		Erosion control	1.00	LS	10,000.00	\$ 10,000.00
						\$ 240,200.00

Notes:

1 Larson Construction is a Union Contractor.

2 Utility conflicts to be resolved with help from the Prime Contractor

3 All items in Base Bid are tied unless discussed prior to bid

Signed:

Tim MacLaughlin-Barch, Vice President, Operations

Date: 3/20/2024

Accepted:

Date:

Guidelines for Applicant

Complete this form for each bridge structure you are submitting a grant application for. Provide any additional documents not requested on application checklist to substantiate your points, including actual deeded easements. Actual

	Category	Points	Points
1	Condition of the Structure (max of 10 points)		
1	Has a certified bridge inspection report that supports the project & demonstrates need (see	10	10
2	example, must provide copy of report by August 1 for 2024 only j		
2	Computation with DNR Water Mgmt Specialist has occurred & permit is likely, if needed	1	1
-	Consultation with Divide already permitted	3	
3	Funding (maximum points 2) Are other funds already committed?		
	50% or greater from other funding source(s)?	2	
	11% - 49% from other funding source(s)?	1	NOVELO ASSINGUE AVAILAD DE RABBIER
L	Length of Written Easements or Land Use Agreement (max points 5)(ch.		
	23.09(26)(am)1 W1 Stats)	5	
	10 or more year deeded easement on private land or other public land, for all portions of	5	-5-
	that trail to the nearest road on each side of the bridge 3-9 year deeded easement on private land or other public land, for <u>all portions of that</u>	4	Not Deed
_	trail to the nearest road on each side of the bridge 10 or more year deeded easement on private land or other public land, for just the bridge	3	
	site	2	
	10 or more year land use agreement (LUA, not deeded) on private land or other public	1	1
	land	0	
_	3-9 year land use agreement (LOA, not decided) on pirtue tand to replaced?		
	Miles Impacted – How many miles will need to refore in the structure is needed of Miles Impacted of Measured from nearest intersection on both sides of the bridge. (max 4 points)		
	Less than 20 miles	1	
	20 miles or more	3	<u> </u>
	No other snowmobile trails connect. Explain:	4	
-	DEDUCTIONS		
F	6 County Active Project Deduction (maximum deduction 1 point) A snowmobile active		
	project is one that has exceeded it's initial grant period.		
	Two or more active projects - deduct 1 point	-1	N/M
	GRAND TOTAL	Ĺ	XXXX
			15

Comments/Notes:

























July 23, 2024



Barron County Sheriff's Department Barron County Justice Center 1420 State Hwy 25 North Room 1200 Barron, WI 54812-3007

Eau Claire Office 1802 Warden Street Eau Claire, WI 54703 608.828.1011 P

Trail Bridge #12417 Inspection and Load Rating

CORRE, Inc. (CORRE) has completed the inspection and load rating of the trail bridge over Fourmile Creek at latitude and longitude of 45.340019 N, 91.873274 W.

INSPECTION

Painted Steel Girders All paint gone with some minor pitting / corrosion but no section loss. Both rows of girders are spliced at mid-span and no sign of defects. <u>Fair condition</u>

Painted Steel Diaphragms All paint is gone with pitting / corrosion but no section loss. Fair condition

Timber Abutments: Timber only visible at 6-inches each side of bearing locations. No sign of crushing at the girder bearing. <u>Fair condition</u>

Abutment Slopes: gravel / soil, no erosion or scour. Fair condition.

Timber Deck: 3" x 8" timber plank. Timbers are aged and have minor decay / white fungus on underside throughout. Some planks are spongy and should be replaced. <u>Poor condition</u>

Timber pile: Piles at all four piers have failed. Holes through entire section of most. All have decay and crushing. <u>Poor condition / Replace</u>

Railing: light steel tubing with welded wire fence between posts. Posts are rusty; connections are anchored to timber deck plank. <u>Poor condition</u>

Recommendations:

Replace entire structure (salvage the existing steel girders).

Inspection Frequency:

Pedestrian/trail bridges that do not carry highway traffic are not under the NBIS regulations. WisDOT does recommend inspections on a 48-month cycle for these structures.

CORRE recommends this structure be inspected on a 12-month inspection cycle.

LOAD RATING

Based on our review of the structure and discussions with the client, it is desirable to salvage the existing steel girders and place them on new timber piers. All existing piers would be removed, and 2 new piers are proposed to be installed, resulting in a 3-span configuration. The proposed span lengths would be 20.0', 22.5', 20.0'. New panel decking and timber railing would be installed.

Under the proposed conditions noted above, the existing steel girders will have the capacity to handle a truck load of 24 tons. It should be noted that prior to construction, analysis of the existing steel splice in both girders should be performed to confirm adequacy, which is outside the scope of effort for this load rating.

SUMMARY

CORRE appreciates the opportunity to provide these services. If you have any questions or would like to discuss further, please feel free to contact me at 715.579.0325 or Mark Pilgrim at 715.726.9631.

Sincerely,

CORRE, Inc.

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Eric Price, PE Structural Services Director

Enc: Inspection Summary, Load Rating Calculations

nark Pilgum

Mark Pilgrim, 9501 Bridge Inspector





South Approach Looking North



West Profile



Typical Girder condition



Typical Timber abutment (W)





Typical Timber abutment (E)



Typical Timber abutment (E)



Typical pile condition at pier



Typical bearing at pier





Typical timber pier cap



Timber and girder condition at contact.

Client: Barron County Sheriff	
Project Description: Load Rating	ANSWERS. CORRE
Structure No.: 12417	SOLUTIONS.
Rated By: SAE Checked By: ETP	RESULTS EL 🚟
Date: 7/23/2024	
File Name: https://correinc.sharepoint.com/sites/Projects/NW Region/202403.025_2024 And Span - 3 SPAN - MIDDLE SPAN - Snowmobile-Rating (LFR)-12417.xlsx]program	cillary Bridge Inspections/300_Design/309_Structures/Ratings/12417/[Steel Girder-Multi
AASHIULFR SIEEL GI	ADER RATING
#VALUE!	
Structure Information:	
Span Length =	Span 2 controls 22.50 feet
Clear Width =	12.50 feet
Location of Beam (Interior or Exterior) =	Exterior feet
Number of Girders =	2 total
Girder Spacing =	6.00 feet
Overnang Width =	3.00 feet
Rail Width =	0.25 feet
Deck Thickness =	6 00 inches
Depth of Wearing Surface =	0.00 inches
Average Haunch Depth =	0.00 inches
Deck Configuration – Composite? No	Plank (wide face bearing on support)
, ,	
Overlay Thickness = Type = None	0.00 inches
Overburden Thickness = Type = None	0.00 inches
Weight of Rail =	27.0 lbs/foot
Location of M1 -	25.00 foot
LUCation of M1 = Sta Specific Hwy Bildges That T0.46.1.1	23.00 leet
DL Moment @ location of M1 =	1 34 ft-kips
	1.04 11-10-5
Unbraced Length of Compression Flange of Girder =	15.00 feet
Compressive Strength of Concrete =	N/A psi
Area of Longitudinal Steel, at Top of Slab, at Location of Positive Moment =	N/A inches^2/foot
Yield Strength of Reinforcing Steel =	N/A psi
Modular Ratio = E_s/E_c =	N/A
Girder/Beam Properties:	
Yield Strength =	<u>30</u> ksi
Snape =	LK 18X55
Bollom Flange:	6 00 inches
Width =	0.00 inches
Bottom Flance Cover Plate:	0.44 IIICHES
Width =	0.00 inches
Height =	0.00 inches
Web:	
Width = <u>Assumed Section Loss</u>	0.46 inches
Height = 0%	17.13 inches
Top Flange:	
Width =	6.00 inches
Height =	0.44 inches
Top Flange Cover Plate:	
Width =	0.00 inches
Height =	0.00 inches
i ransverse Stiffener Spacing =	N/A
Longiludinal Stiffener Used =	NO

Total Depth, d =				18.00 i	nches
Web Thickness, $t_w =$				0.46 i	nches
Compression Flange Thickne	ss, t _f =			0.44 i	nches
Compression Flange Width, b	o _f =			6.00 i	nches
Weak Axis Radius of Gyration	n, r _y =			1.15 i	nches
Area, A =				15.93 i	nches^2
Strong Axis Section Modulus,	S _x =			88.40 i	nches^3
Web Thickness, $t_y =$ 0.000Compression Flange Thickness, $t_f =$ 0.44 iCompression Flange Width, $b_f =$ 0.44 iCompression Flange Width, $b_f =$ 0.001Weak Axis Radius of Gyration, $r_y =$ 1.15 iArea, A =15.93 iStrong Axis Section Modulus, $S_x =$ 88.40 iStrong Axis Moment of Inertia, $I_x =$ 795.60 iLive Load Distribution:1.) Interior Girdersa.) For Moment (AASHTO Table 3.23.1)1.500b.) For End Reaction (AASHTO 3.23.2.12)1.500b.) For End Reaction (AASHTO 3.23.1.2)1.0002.) Exterior Girders1.000a.) For Moment(1) AASHTO 3.23.2.3.1.2Distribution Factor =1.125 (0.000b.) For End Reaction (AASHTO 3.23.1.2)0.000b.) For End Reaction (AASHTO 3.23.1.2)1.125 (0.000b.) For End Reaction (AASHTO 3.23.1.2)0.000 0.000b.) For En		nches^4			
Live Load Distribution:					
1.) Interior Girders					
a.) For Moment (AASH1)	J Table 3.23.1) S/4 00 -			1 500	
b) For End Reaction (AA	SHTO 3 23 1 2)			1.500	
Distribution Factor =	(01110 0.20.1.2)			1.000	
2.) Exterior Girders					
a.) For Moment	1.0				
(1) AASHTO 3.23.2.3 Distribution Factor =	.1.Z			1 125 (aoverne)
(2) AASHTO 3.23.2.3	.1.5			1.125 (governs)
Distribution Factor =				0.000	
b.) For End Reaction (AA	SHTO 3.23.1.2)				
Distribution Factor =				1.125	
			Max Moment	Max Reaction	
Non-Composite Dead Loads	0 125 kH -		(ft-kips)	(kips)	
Deck =	0.135 Klf =		8.54	1.52	
Girders =	0.000 klf = 0.054 klf =		3 4 3	0.00	
Secondary Members =	0.020 klf =	override	1.27	0.23	
=	0.209 klf =	10.7	10.70	2.35	
Composite Dead Loads					
Overlay =	0.000 klf =		0.00	0.00	
Rail Load =	0.027 klf =		1.71	0.30	
Snow (12" Depth) =	0.123 klf =	<u>override</u>	7.78	1.38	
	0.150 klf =	1.1	7.70	1.69	
Live Loads		<u>override</u>			
H20 Loading =		132.9	132.90	35.00	
Pedestrian Loading =	85 psf =	49.5	49.50	11.95	
Impact, I = 50/(Span + 125) ≤	: 1.30 =	3.8.2.1		1.000	<u>override</u> 1.000
Controlling Live Load (Distribu	uted) =		74.8 ft-kips	19.7 kips	
M1 =	Std Spec for Hwy Bi	ridges 17th 10.48.1.1	44.2 ft-kips		
Design Land Fasters					
Factor Applied to Dead Load	. LF _d =	Table 3.22.1A		1.3	
Factor Applied to Composite	Dead Load, LF ₂ =			1.3	
Factor Applied to Live Load, I	_F₁ =			2.17	

COMPUTATION OF SECTION PROPERTIES

Properties of Noncomposite Section Resisting Dead Loads: Moment of Inertia, I = Distance from Bottom of Beam to N.A., Yb = Distance from Torn of Beam to N.A. Yb =	795.60 inches^4 9.00 inches
Section Modulus to Bottom of Beam. Sb =	9.00 inches 88.40 inches^3
Section Modulus to Top of Beam, St =	88.40 inches^3
Properties of Composite Section Resisting Dead Loads:	
Moment of Inertia, I =	795.60 inches^4
Distance from Bottom of Beam to N.A., Yb =	9.00 inches
Distance from Top of Beam to N.A., Yt =	9.00 inches
Section Modulus to Bottom of Beam, Sb =	88.40 inches^3
Section Modulus to Top of Beam, St =	88.40 inches^3
Properties of Section Resisting Composite Dead Loads (3n):	
Moment of Inertia, I =	#VALUE! inches^4
Distance from Bottom of Beam to N.A., Yb =	#VALUE! inches
Distance from Top of Beam to N.A., Yt =	#VALUE! inches
Distance from Top of Deck to N.A., Ydeck =	#VALUE! inches
Section Modulus to Bottom of Beam, Sb =	#VALUE! inches^3
Section Modulus to Top of Beam, St =	#VALUE! inches^3
Section Modulus to Top of Deck, Sdeck =	#VALUE! inches^3
Properties of Section Resisting Live Loads (n):	
Moment of Inertia, I =	795.60 inches^4
Distance from Bottom of Beam to N.A., Yb =	9.00 inches
Distance from Top of Beam to N.A., Yt =	9.00 inches
Distance from Top of Deck to N.A., Ydeck =	15.00 inches
Section Modulus to Bottom of Beam, Sb =	88.40 inches^3
Section Modulus to Top of Beam, St =	88.40 inches^3
Section Modulus to Top of Deck, Sdeck =	0.00 inches^3

ANALYSIS OF NONCOMPOSITE SECTION

etermine Section Designation:		10.48.1	10.48.2	10.48.4
			Braced	
		Compact	Noncompact	Unbraced
		Section	Section	Section
		Requirements	Requirements	<u>Requirements</u>
b'/t =	6.86	11.86	12.70	N/A
D/t _w =	37.23	111.02	88.91	105.37
$D/t_w + 9.35(b'/t) =$	101.34	N/A	N/A	N/A
L _b =	180.00	119.65	97.22	N/A
$I_{yc}/I_y =$	0.37	N/A	N/A	0.1 to 0.9
Classification of Section =				Noncompac

Bracing Classification of Section =

Unbraced

oment Capacity of Section		
$M_p = F_y Z =$	(10-92)	199.6 ft-kips
$M_y = F_y S =$	(10-98)	221.0 ft-kips
Determine M _r :		
L _p =		95.00 inches
L _r =	(10-103f)	174.85 inches
L _b =		180.00 inches
$D_c/t_w =$		18.61
$\lambda/F_{y}^{0.5} =$		88.91
18,250/F _y =		105.37
For $D_c/t_w \le \lambda/F_v^0.5$ or with Longitudinally Stiffened We	ebs:	
M _r =	(10-103c)	143.1 ft-kips
For Lambda/F _v ^0.5 < D₂/t _w ≤ 18.250/F _v ^0.5:		
For $L_h \leq L_n$:		
$M_r = M_{v} =$	(10-103d)	N/A ft-kips
For $L_z \ge L_h > L_n$:	()	
$M_r =$	(10-103e)	N/A ft-kips
For $L_b \ge L_c$:	()	
$M_r =$	(10-103g)	104.27 ft-kips
Governing M _r =		143.1 ft-kips
Determine R _b :		
Depth of Web =		17.13 inches
Depth of Web in Compression, D_c =		8.56 inches
Area of Compression Flange, A _{fc} =		2.63 inches^2
Lateral Torsional Buckling Moment or Yield Moment, N	1 _r =	1,717,049 inch-lbs.
Section Modulus for Noncomposite Section =		88.40 inches^3
$\lambda =$		15,400.00
R _b =	(10-103b)	1.00
Governing Moment Capacity of Section, M _u =		143.1 ft-kips
ear Capacity of Section:	F _y of Web =	30 ksi
$V_u = CV_p$	(10-113)	
C =		1.00
$V_{p} = 0.58F_{y}^{*}d^{*}t_{w} =$	(10-115)	5,102.8 kips
Shear Capacity of Section, V _u =		5,102.8 kips

STRUCTURE RATINGS

Ratings Based o Available M _{II+} Inventory Rat	91.7 ft-kips 0.74					
Inventory Rat Operational F	ting = Rating =					H 14.7 H 24.5
	Vehicle	Weight (tons)	Operating RF	Capacity	Recommended	
=	H20	20	1.23	24.5 Tons	24 Tons	
Ratings Based o	<u>n Shear:</u>					
Available V _{II+i}	3,921.2 kips					
Inventory Rat	ting Factor =					119.50
Inventory Rat Operational F	ting = Rating =					H 2390.1 H 3983.4

GOVERNING RATINGS

Structure shall be limited to a maximum vehicle load of =

24 Tons